
Physical Test 8.0 Crack BEST

the mechanical properties of underground rock mainly include mechanical strength and ductility. the strength of underground rock is usually determined based on the compression test. the collapse limit is the most direct measure of the mechanical strength of underground rock. however, the collapse limit is affected by the tensile deformation capacity of rock. therefore, the collapse limit does not accurately reflect the actual mechanical strength of the rock. for most types of underground rock, the fracture strength is the most direct measure of the mechanical strength of underground rock, and the fracture strength of underground rock is directly proportional to the compressive strength. the fracture strength of underground rock refers to the average load bearing capacity of rock when being subjected to large-scale deformation. the collapse limit is the most direct measure of the mechanical strength of the rock. therefore, the collapse limit is not suitable for the measurement of the mechanical strength of a rock mass which is in a stressed state. it is the most direct and sensitive measure of the fracture strength of the rock.

1. the collapse limit of the rock mass increases gradually with the loading. the compression load decreases gradually after the collapse of the rock mass. the fracture strength is the first load to break the rock mass. when the collapse limit is reached, the rock mass is fractured into two parts and the fracture strength can be calculated by the fracture load.
2. the initial collapse of the rock mass is accompanied by a certain amount of elastic deformation. the collapsed part of the rock mass is relatively soft. when the fracture load is applied, the rock mass is crushed and finally broken into two parts.
3. the fracture strength of the rock mass is calculated by the area between the loading curve and the load-fracture curve. the loading curve is the curve of rock mass stress under repeated loading. the load-fracture curve is the curve of fracture load under constant loading. the area between the two curves is the fracture strength of the rock mass. in order to improve the accuracy of the fracture strength, the fracture strength is commonly measured by the fracture load. the test method is as follows: step 1. a load is applied to the rock mass. when the collapse limit is reached, the fracture load is recorded. step 2. the fracture load is applied to the rock mass again. step 3. the above two fracture loads are added to calculate the fracture strength of the rock mass.

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figure 15 shows progressive damage to the interior wall of the tunnel model after excavation in physical model tests. when tunnels with different sections were excavated under the same initial stress, in addition to the different processes governing the length and depth of cracks in the surrounding rock, the position of the point of failure on the inner wall also differed. it can be seen, from figure 15, that the position of spalling failure of tunnel t1 is mainly concentrated at the foot of the side wall, while that in t2 is mainly concentrated at the midheight of the side wall, proving that the section of the underground tunnel influenced the position of failure therein: not only that, but the degree of damage to the interior walls of the two tunnels also varied with cross-sectional shape. continuous spalling occurred at the foot of the horseshoe tunnel t1. discontinuous spalling occurred at the midheight of the side wall of tunnel t2. the spalling depth of tunnel t1 was much larger than that of tunnel t2. as a result, when the tunnels with different cross-sectional shapes were excavated and loaded under the same stress, the damage to the inner wall of the horseshoe tunnel was more severe than that of the circular tunnel. the layout of monitoring lines is an important aspect of monitoring the whole process of tunnel failure. according to the field observation, the middle of the model tunnel is the most damaged and the strain is the largest. therefore, 11 measurement points were arranged in the middle of the left wall of the model tunnel. while building the physical model, strain gauges and an electrical resistance grid technique were placed in the rock surrounding the tunnel to monitor the whole process of tunnel failure. two strain gauges were arranged at each measuring point (specification: 215mm colloidal strain gauge). one of the strain gauges measures the strain in the radial direction, and the other measures the strain in the circumferential direction. in order to monitor the strain at cracking process, the spacing of the strain gauges arranged near the wall is small (figure 5). to monitor the process of crack propagation, an electrical resistance grid technique was arranged at the midheight of the side wall and the middle of the roof. the length of the electrical resistance grid is 100mm. as shown

in figure 6, the 21 contacts were placed on the electrical resistance grid. the distance between each contact point is 5mm, and each contact was connected to the lead wire through a 2mm3mm terminal. each of the two adjacent lead wires was connected to an led. when a crack was not generated, the electrical resistance grid was not broken, and the led was not short circuited. after tunnel cracking was generated, the electrical resistance grid was broken in the corresponding position(s) and the led illuminates. therefore, the process of crack propagation could be monitored by led illumination sequences. 5ec8ef588b

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